

Inundation Modeling, Breach Parameters, and Consequences (Introduction)

Best Practices in Dam and Levee Safety Risk Analysis
Part C – Consequence Estimating

Last modified June 2017, presented July 2018



US Army Corps
of Engineers®



Key Concepts

- Risk management involves consequence management
- Scalable approach based on goals of analysis
 - Initial characterization vs. prioritization vs. risk reduction
- Life risk is paramount
 - Understanding human factors is critical
- Build the case
 - How many people are exposed?
 - Warning and evacuation considerations
 - Flood characteristics?
 - Breach parameters
 - Inundation modeling
- Embrace uncertainty



Definitions

- Consequence
 - Direct vs. Indirect
- Life Loss
 - Population at risk
 - Exposed/threatened population
 - Fatality rate
- Economic
- Environmental
- Cultural



Essential Elements of Life Loss Estimate

- How many people are exposed to the flooding?
 - Initial distribution of people
 - Redistribution through evacuation
- How severe is the flooding?
- Are the people in a structure that can withstand the flooding?
- Will some of the people subjected to flooding die?



Empirical vs. Simulation Models

Empirical:

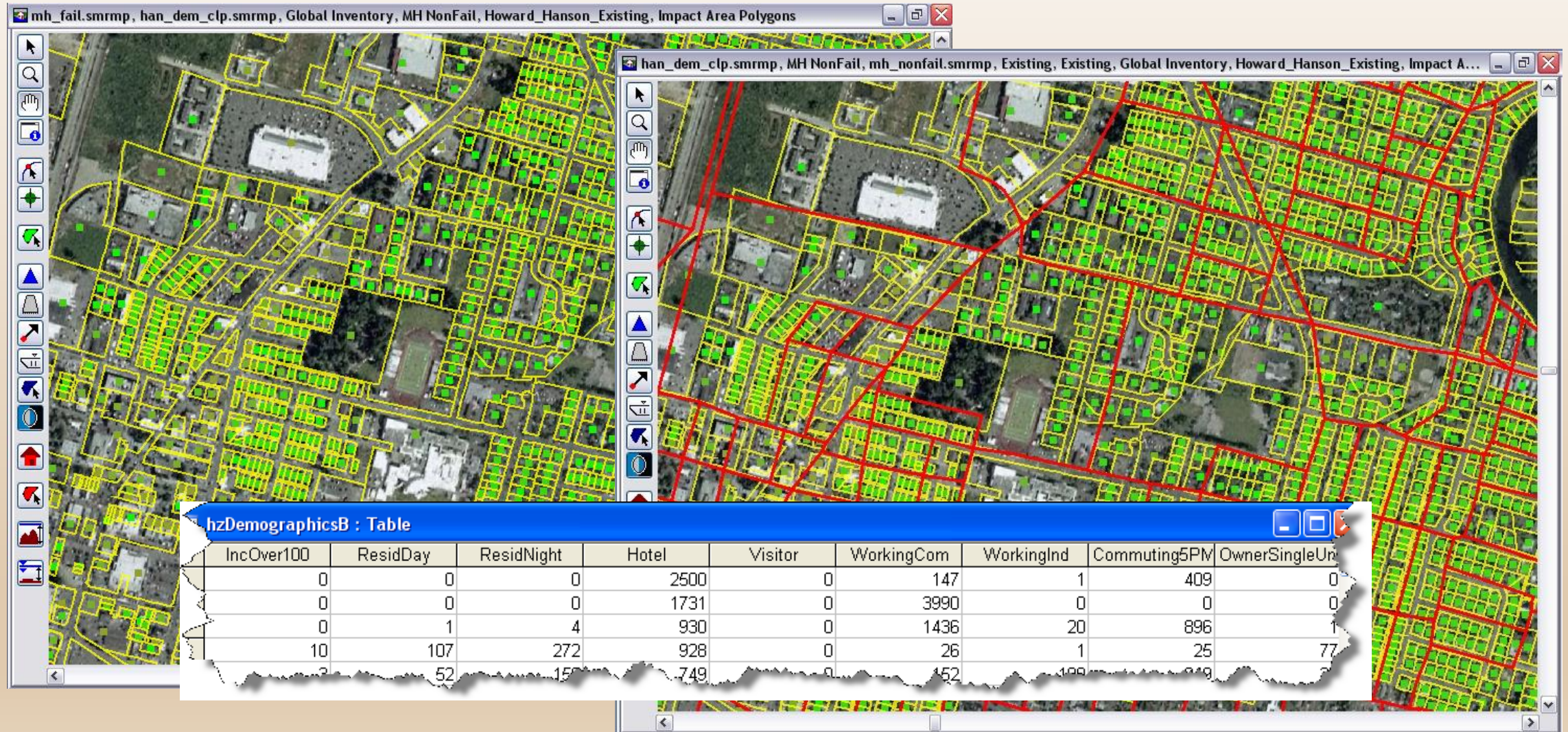
- Groups of PAR evaluated in aggregate
- Fatality rates ranges reflect evacuation rate assumptions – evacuation is not explicitly modeled
- Relevant parameters are warning time and the intensity of flooding

Simulation:

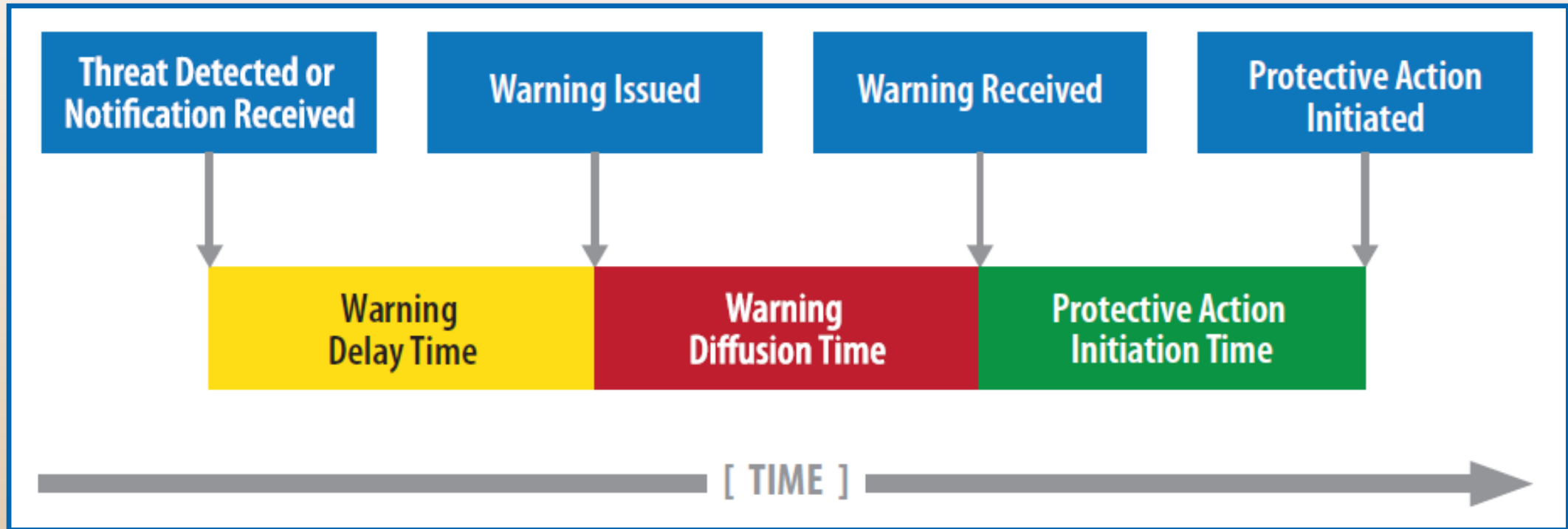
- Tracks movement of people and movement of water – evacuation is explicitly modeled
- Each individual or defined group is evaluated separately
- Fatality rates can be applied to PAR which exceed critical flood parameter thresholds

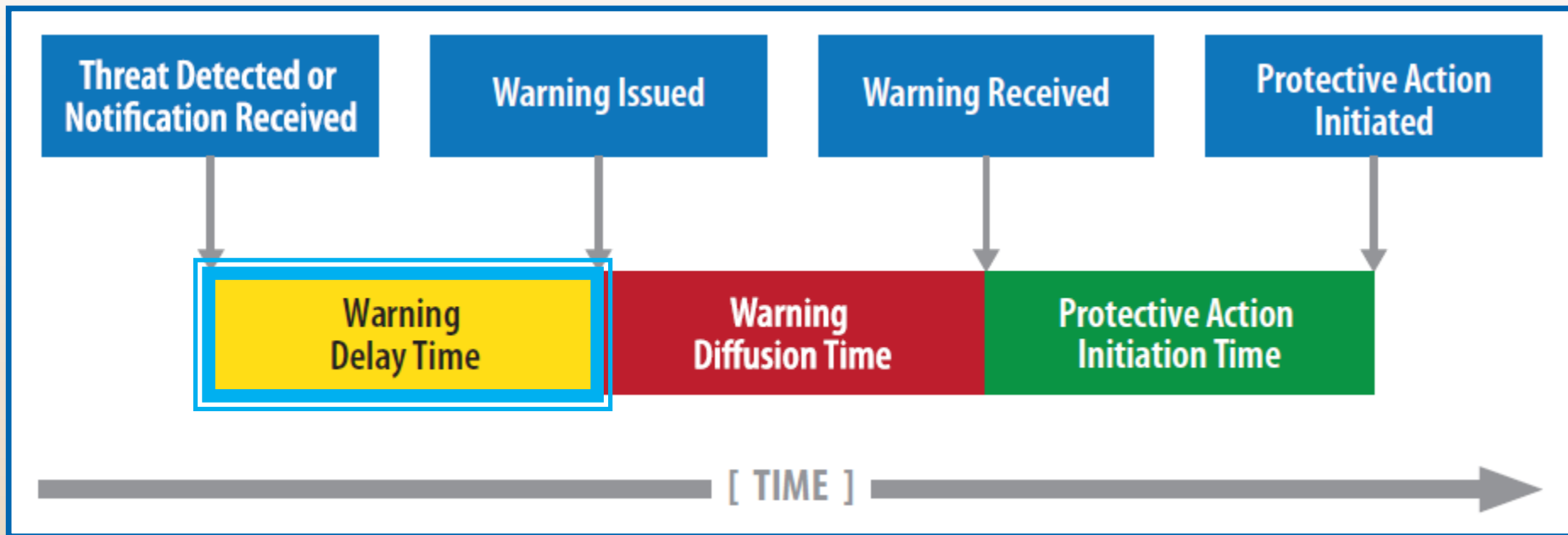


Initial Distribution of People

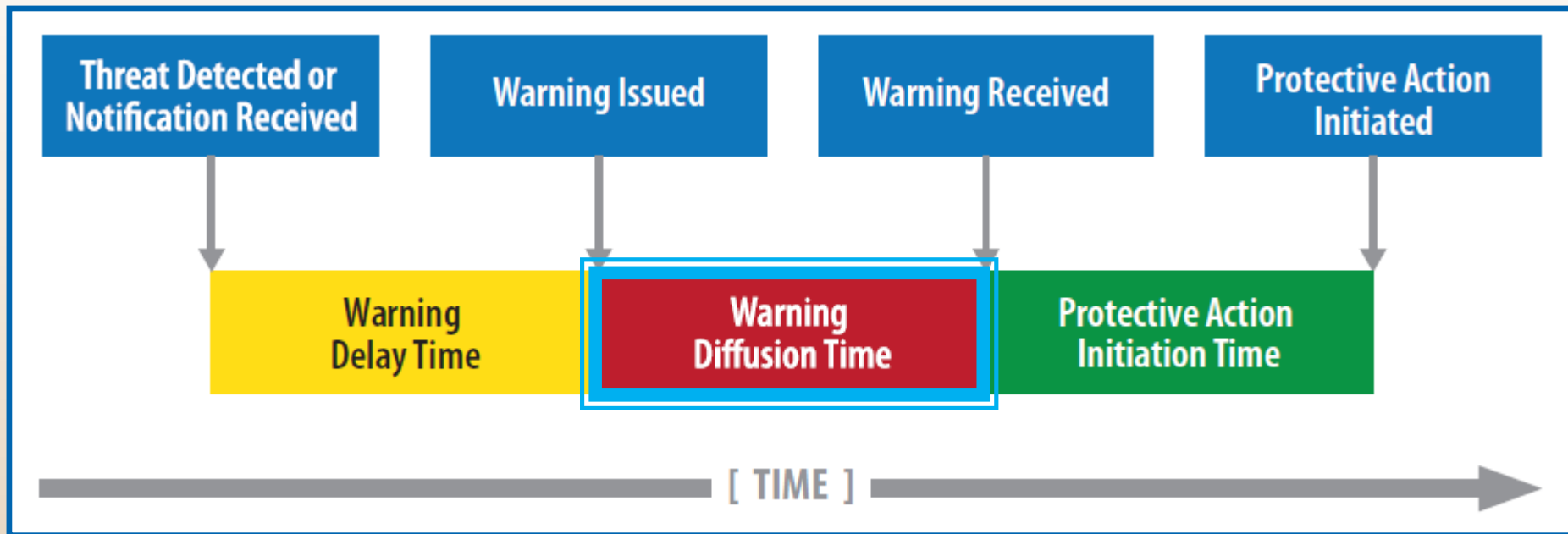


Redistribution of People (Evacuation Effectiveness)

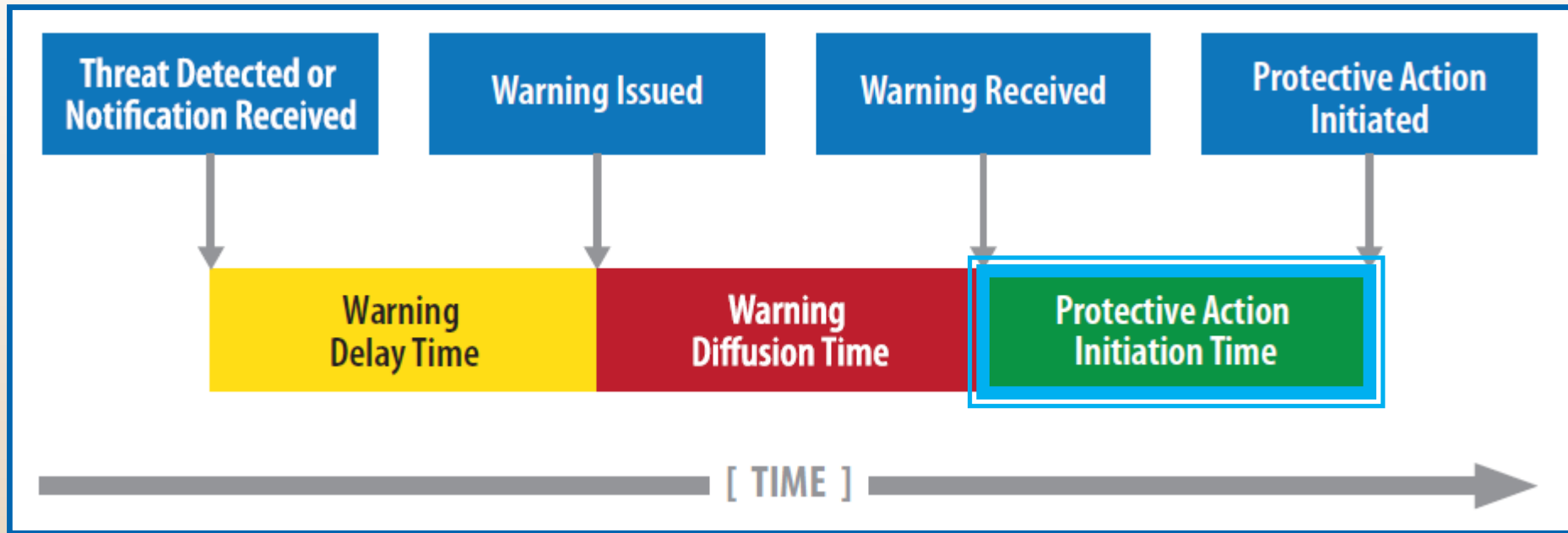




- Standard Warning Plan and Standard Operating Procedures are *Written Down*
- Warning Thresholds Are in Place
- SOP Drills Are Conducted
- Responsibilities are Identified and Clearly Define Authority To Issue Warnings



- Number and mix of warning channels
- Frequency of distribution
- Ability to wake people up
- Modern technologies



- Message content and style

Message Content

- **The single most important thing that an emergency manager can do** to motivate effective public protective action is to provide the best emergency messages possible.

SOURCE: say who the message

THREAT: describe the flooding

LOCATION: state the impact area

GUIDANCE/TIME: tell people

EXPIRATION TIME: tell people when the alert/warning expires and/or new information will be received

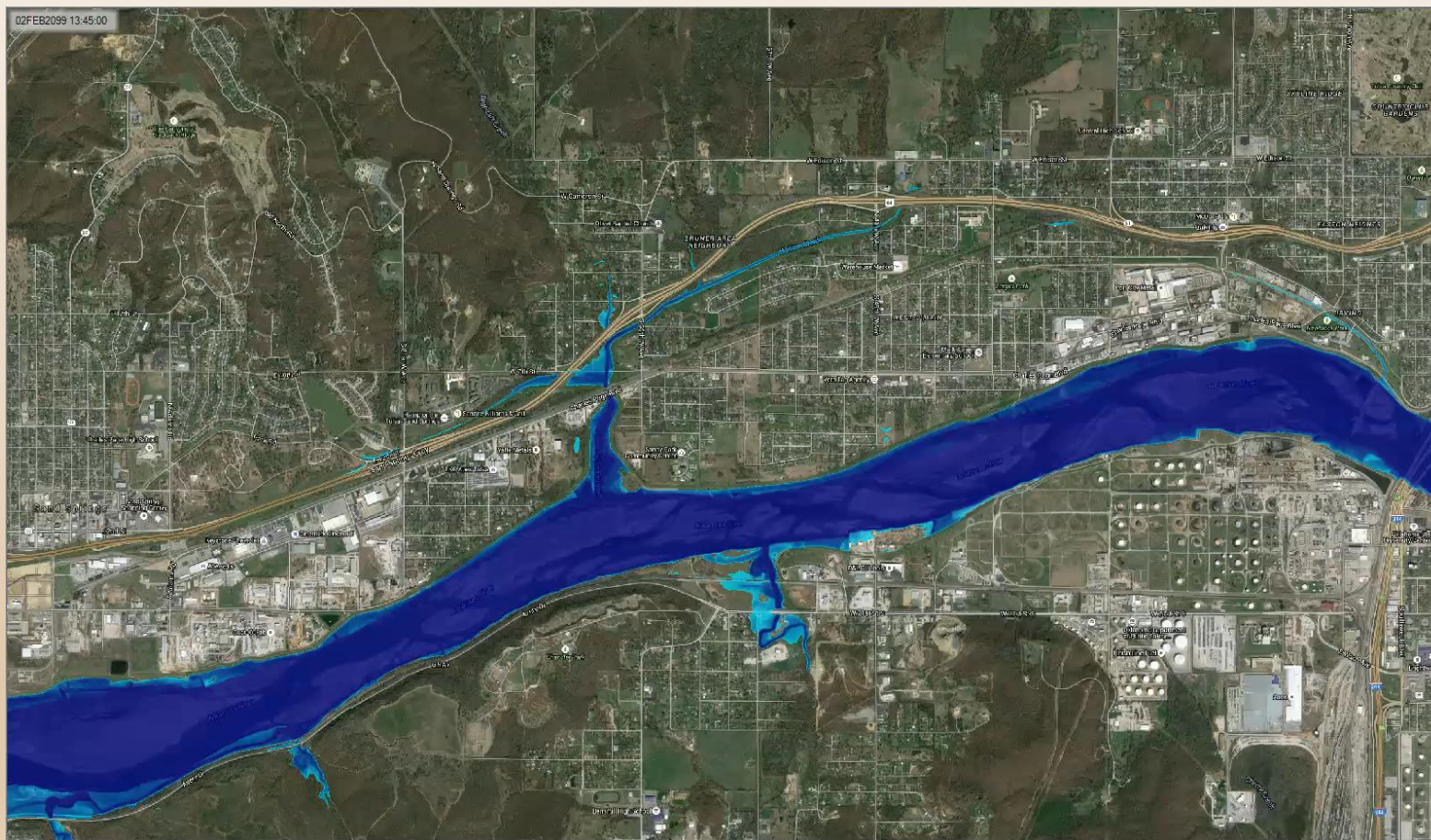
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Flood Severity

- Depth
- Velocity
- Depth * Velocity
- Arrival time
- Extents



Key Concepts For Inundation Modeling

- Scenario
 - Pool or stage elevation and hydrology
 - Breach or Non-breach
 - Failure mode
- Breach parameters
- Terrain
 - 1d vs. 2d
- Initial conditions
- Incremental/coincident flows

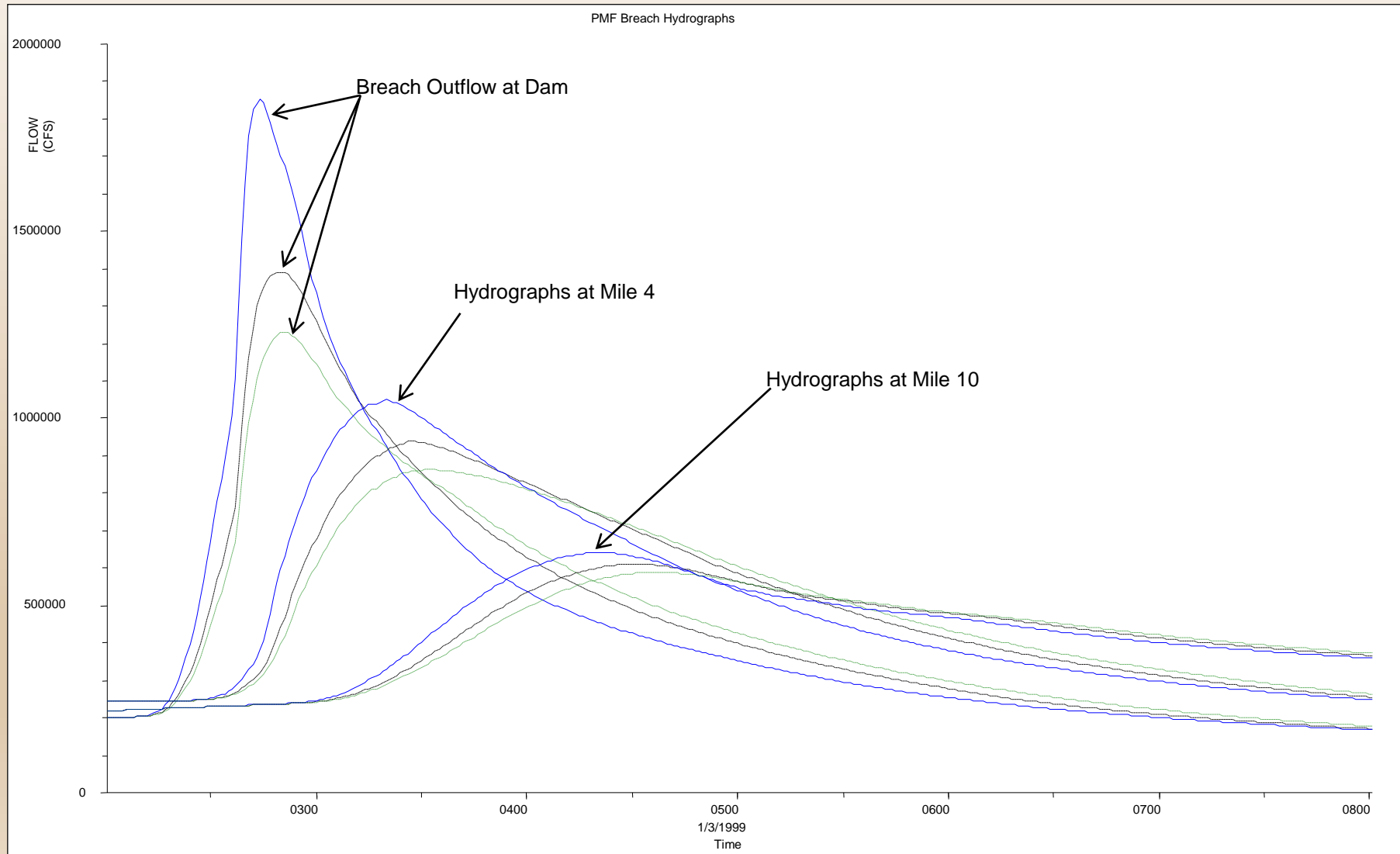


Key Concepts for Understanding and Selecting Breach Parameters

- Breach parameters can impact the following flood characteristics
 - Depth
 - Velocity
 - Arrival time (and therefore warning time)
 - Consequences
 - Life loss, direct damage, repair costs, etc
- Sensitivity analysis should be performed prior to detailed breach parameter analysis
 - Adopt scalable approach based on outcome
- Tradition empirical equations are based on dam breach cases

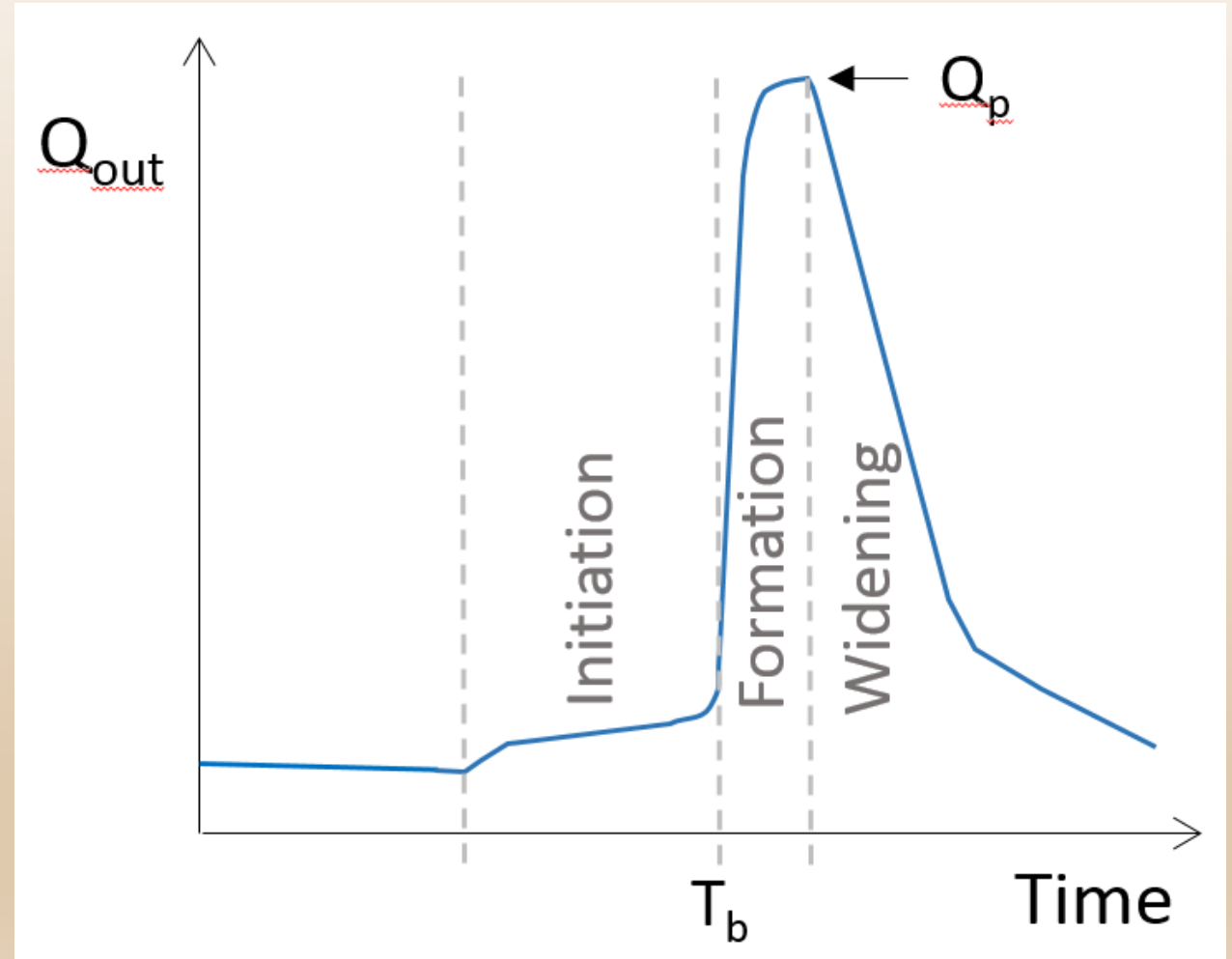


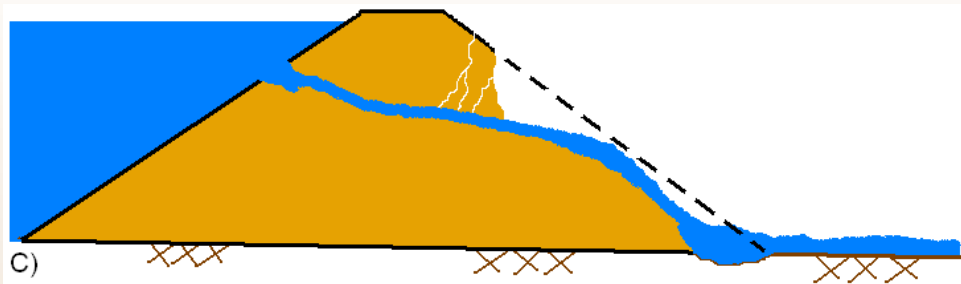
Does it Matter? Depends on downstream terrain, location of PAR and other factors..



Breach Parameters Definitions

- Breach initiation
 - Typically not included in hydraulic model
- Time of breach (T_b)
- Breach formation
- Breach widening

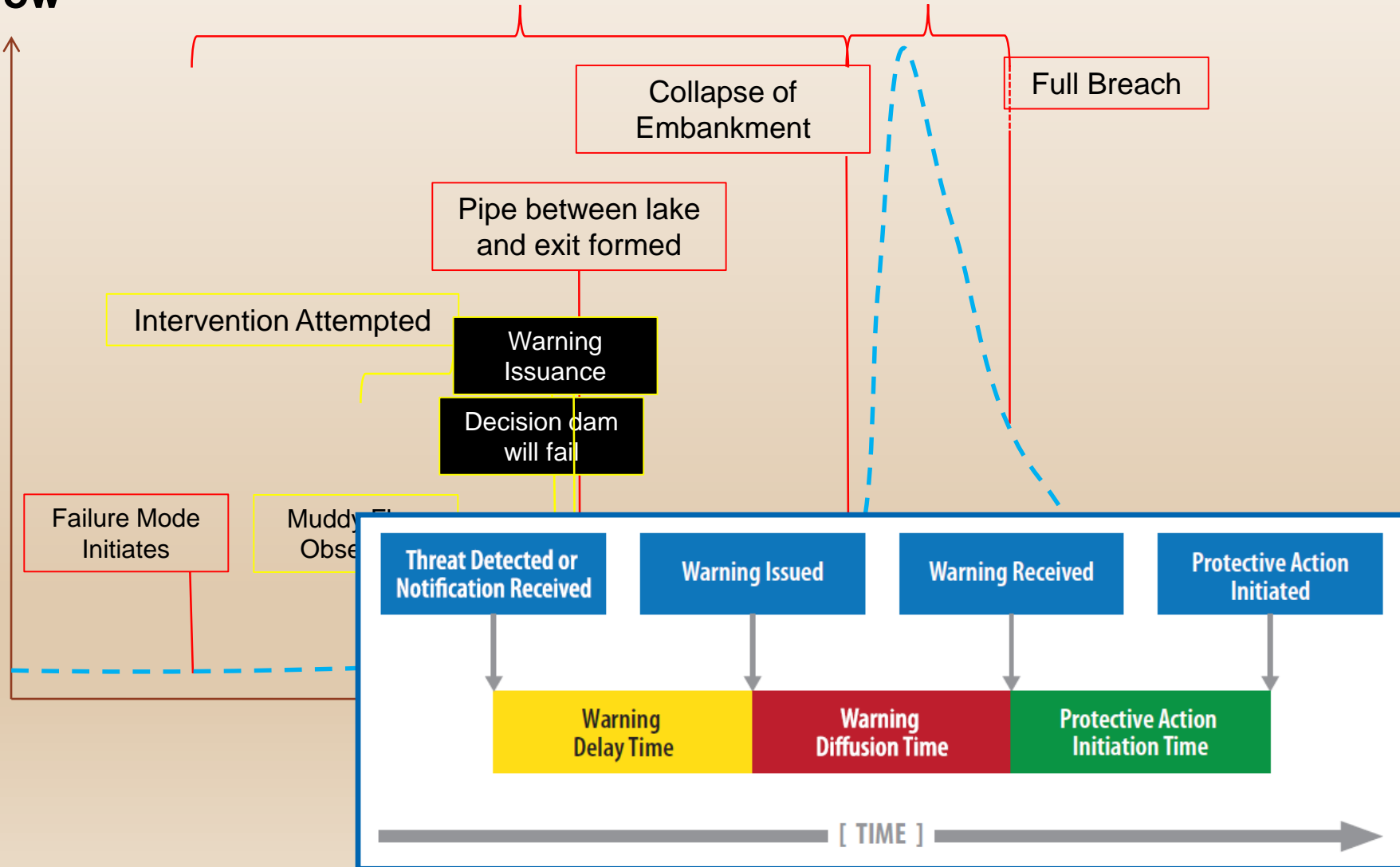
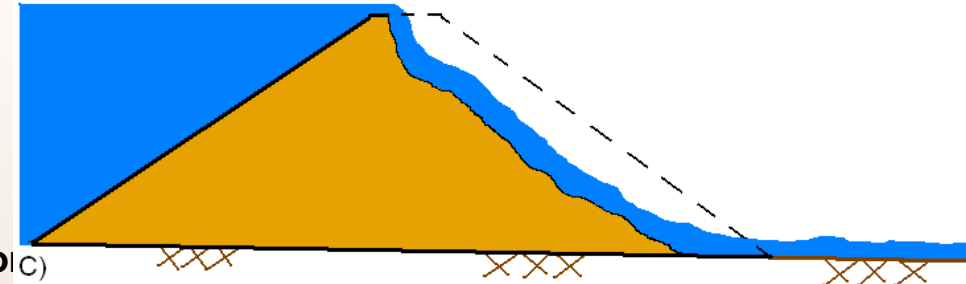




Flow

Initiation Time

Breach Flow (MacDonald, Froehlich, V.T. & G.)



Options for Estimating Breach Parameters

- User defined
 - Historic data, empirical equations, site specific assumptions, etc
- Simplified physical breaching
 - Velocity vs. erosion rate
- Coupled embankment erosion and hydraulic model

Reference	Number of Case Studies	Relations Proposed (S.I. units, meters, m ³ /s, hours)
Johnson and Illes (1976)		$0.5h_d \leq B \leq 3h_d$ for earthfill dams
Singh and Snorrason (1982, 1984)	20	$2h_d \leq B \leq 5h_d$ $0.15 \text{ m} \leq d_{\text{outtop}} \leq 0.61 \text{ m}$ $0.25 \text{ hr} \leq t_f \leq 1.0 \text{ hr}$
MacDonald and Langridge-Monopolis (1984)	42	Earthfill dams: $V_{er} = 0.0261(V_{out}^* h_w)^{0.769}$ [best-fit] $t_f = 0.0179(V_{er})^{0.364}$ [upper envelope] Non-earthfill dams: $V_{er} = 0.00348(V_{out}^* h_w)^{0.852}$ [best fit]
FERC (1987)		B is normally 2-4 times h_d B can range from 1-5 times h_d $Z = 0.25$ to 1.0 [engineered, compacted dams] $Z = 1$ to 2 [non-engineered, slag or refuse dams] $t_f = 0.1$ - 1 hours [engineered, compacted earth dam] $t_f = 0.1$ - 0.5 hours [non-engineered, poorly compacted]
Froehlich (1987)	43	$\bar{B}^* = 0.47 K_o (S^*)^{0.25}$ $K_o = 1.4$ overtopping; 1.0 otherwise $Z = 0.75 K_c (h_w^*)^{1.57} (\bar{W}^*)^{0.73}$ $K_c = 0.6$ with corewall; 1.0 without a corewall $t_f^* = 79(S^*)^{0.47}$
Reclamation (1988)		$B = (3)h_w$ $t_f = (0.011)B$
Singh and Scarlatos (1988)	52	Breach geometry and time of failure tendencies $B_{\text{top}}/B_{\text{bottom}}$ averages 1.29
Von Thun and Gillette (1990)	57	B , Z , t_f guidance (see discussion)
Dewey and Gillette (1993)	57	Breach initiation model; B , Z , t_f guidance
Froehlich (1995b)	63	$\bar{B} = 0.1803 K_o V_w^{0.32} h_b^{0.19}$ $t_f = 0.00254 V_w^{0.53} h_b^{(-0.90)}$ $K_o = 1.4$ for overtopping; 1.0 otherwise



Numeric Modeling Options for Estimating Breach Parameters

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Process	WinDAM B/C	DL Breach	HR BREACH	NWS BREACH
River Hydraulics	No	N	N	N
Breach Flow	Yes	Y	Y	Y
Internal Hydraulic Routing	N	N	Y	N
Tailwater Submergence	Y	Y	Y	Y
Piping Initiated	Y	Y	Y	Y
Overtopping Initiated	Y	Y	Y	Y
River Erosion and Stability Failure Initiated	N	N	N	N
Headcut	Y	Y	Y	N
Breach Widening	Y	Y	Y	Y
Breach Deepening	Y	Y	Y	Y
Foundation Scouring	N	Y	N	N
Mass Wasting	Y	Y	Y	Y
Surface Erosion by Sediment Transport	N	Y	Y	Y
Sediment Volume	N	Y	Y	Y
Surface Protection Removal	Y	N	Y	Y
Composite Material Zones	N	Y	Y	Y



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Building type	Partial damage	Total damage
Wood-framed		
unanchored	$v^*d \geq 2 \text{ m}^2/\text{s}$	$v^*d \geq 3 \text{ m}^2/\text{s}$
anchored	$v^*d \geq 3 \text{ m}^2/\text{s}$	$v^*d \geq 7 \text{ m}^2/\text{s}$
Masonry, concrete & brick	$v \geq 2 \text{ m/s}$ & $v^*d \geq 3 \text{ m}^2/\text{s}$	$v \geq 2 \text{ m/s}$ & $v^*d \geq 7 \text{ m}^2/\text{s}$



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Ranges of fatality rates and life loss estimates are required for the empirical approach

Embrace Uncertainty

